

EFFECT OF WIND LOAD ON LOW, MEDIUM, HIGH RISE BUILDINGS IN DIFFERENT TERRAIN CATEGORY

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ABSTRACT: Any Tall building can vibrate in both the directions of along wind and across wind caused by the flow of wind. Modern Tall buildings designed to satisfy lateral drift requirements, still may oscillate excessively during wind storm. These oscillations can cause some threats to the Tall building as buildings with more and more height becomes more vulnerable to oscillate at high speed winds. Sometimes these oscillations may even cause discomfort to the occupants even if it is not in a threatening position for the structural damage. So an accurate assessment of building motion is an essential prerequisite for serviceability. There are few approaches to find out the Response of the Tall buildings to the Wind loads. Wind is a perceptible natural motion of air relative to earth surface, especially in the form of air current blowing in a particular direction. The major harmful aspect which concern to civil engineering structures is that, it will load any and every object that comes in its way. Wind blows with less speed in rough terrain and higher speed in smooth terrain. This paper

presents story drift, story shear, and support reactions occur in different storey Buildings (Low Rise Buildings, Medium Rise Buildings, and High Rise Buildings) due to wind in different terrain category. Totally 12 models for G+5, G+10, G+15 are analyzed using ETABSv9.7.4 package. Present works provides a good source of information about variation in drift, shear are compared as height of model changes and percentage change in drift, shear of same model in different terrain category.

KEY WORDS: Tall Buildings, drift, story shear, ETABSv9.7.4, different terrain category.

I. INTRODUCTION

Wind has two aspects. The first beneficial one is that its energy can be utilized to generate power, sail boats and cool down the temperature on a hot day. The other a parasitic one is that it loads any and every object that comes in its way. The latter is the aspect an engineer is concerned with, since the load caused has to be sustained by a structure with the specified safety. All civil and industrial structures above ground have thus to be designed to resist wind loads. This introductory note is concerning the aspect of wind engineering dealing with civil engineering structures.

IMPORTANCE OF WIND LOADS ON TALL BUILDINGS

Buildings are defined as structures utilized by the people as shelter for living, working or storage. As now a days there is shortage of land for building more buildings at a faster growth in both residential and industrial areas the vertical construction is given due importance because of which Tall Buildings are being build on a large scale.

Wind in general has two main effects on the Tall buildings:

- ✚ Firstly it exerts forces and moments on the structure and its cladding
- ✚ Secondly it distributes the air in and around the building mainly termed as *Wind Pressure*

Sometimes because of unpredictable nature of wind it takes so devastating form during some Wind Storms that it can upset the internal ventilation system when it passes into the building. For these reasons the study of air -flow is becoming integral with the planning a building and its environment

Wind forces are studied on four main groups of building structures:

1. Tall Buildings
2. Low Buildings
3. Equal-Sided Block Buildings
4. Roofs and Cladding

ESTIMATION OF WIND LOAD ON BUILDINGS:

Wind load on a Tall building can be determined by:

1. Analytical Method given in the code IS 875: part 3-1987 which is given by A.G.Davenport. The analytical method is usually acceptable for a building with regular shape and size and is almost based on the geometric properties of the building and without incorporating the effects of the nearby buildings.
2. Secondly the Estimation of Wind Load through Wind tunnel testing with a scaled building model used. In Wind Tunnel Testing for the structural design the Dynamic analysis of the scaled model building is done with Balendra's approach and for the cladding design the Surface

Pressure Measurement analysis with Pressure Measurement system is done.

DYNAMIC ANALYSIS OF WIND FORCES ON TALL BUILDINGS:

This thesis is an attempt to study behavior of the tall buildings under simulated atmospheric boundary layer and to evaluate various experimental and analytical techniques to compute dynamic response and present a detailed comparison. Researchers have laid down several analytical procedures during last few decades. Even though there are several grey areas which need to be addressed to achieve a better prediction of the response, i.e., a designer is interested in storey wise horizontal forces for dynamic analysis and design of structural frames. Hence, emphasize is given to compute the story wise lateral forces on building by analytical procedure and through base forces obtained by Wind tunnel testing on scaled model of building and surrounding terrain.

TYPES OF BUILDINGS

The following 3 buildings are generally considered for the design

Low rise buildings (1 to 6 stories)

Design principles:

- ✚ Low rise housing typically involves residential townhouse/terrace housing or small scale residential apartment buildings
- ✚ Low rise housing will be located typically around the outer edges (beyond 400 metres) of the town centers
- ✚ The typical height for low rise housing is 2-4 storeys

Medium rise buildings (7 to 12 stories)

Design principles:

- ✚ Medium rise housing involves residential apartment buildings, sometimes with cafes or small shops at the ground level

- ✚ Smaller town centers such as Hurlstone Park, Dulwich Hill will have medium rise housing immediately surrounding the main street area
- ✚ Medium rise housing will range from 5-7 storey's
- ✚ Medium rise housing will be limited to 5 storey's in sensitive locations such as interface areas

High rise buildings (13 stories and above)

Design principles:

- ✚ High rise housing comprises both standalone apartment buildings and mixed use buildings that incorporate retail shops and / or commercial uses on the lower levels
- ✚ High rise housing starts from 9 storey's and extends to 25 storey's
- ✚ The lower end of this range will be accommodated mainly within the smaller town centres such as Marrickville, Belmore and Lakemba
- ✚ High rise housing will be located close to the rail station – typically within 200-400m from the station

OBJECTIVES

Following are the main objectives of the work:

1. The main objective of the present work is to study the effect and variation of wind pressure for three categories of buildings Low Rise Buildings, Medium Rise Buildings, and High Rise Buildings for different terrain categories.
2. In the present study the variations of the wind pressure on typical multi-storied Buildings was done by dynamic analysis method is given by the draft code IS-875 part 3 – is studied

3. In the present work, multistory buildings of 6 storey, 11 storey and 16 storey were modeled for different Terrain categories i.e. Terrain categories 1, Terrain categories 2, Terrain categories 3, Terrain categories 4.
4. The analysis of the building has been carried out using ETABSV9.7.4. And the dynamic analysis method.
5. The results from the models (story drift, story shear) are compared in different types of story buildings (low, medium, high rise buildings) for different terrain categories.

SCOPE OF WORK

The scope of the present work includes the study of the Wind load estimation on Tall buildings

1. Based on project, study was undertaken with a view to determine the extent of possible changes in the wind behavior of RC Building Models.
2. RC framed buildings are firstly designed for gravity loads and wind loads.
3. The study has been carried out by introducing symmetrical bare frame building models on different wind terrain categories using dynamic analysis method.
4. The study highlights the effect of wind load in different terrains i.e, Terrain 1, Terrain 2, Terrain 3, and Terrain 4 which are considered in the wind evaluation of buildings.
5. The study emphasis and discusses the effect of wind load for 6 storey, 11 storey, 16 storey Buildings are considered.
6. The entire process of modeling, analysis and design of all the primary elements for all the models are carried by using ETABS 9.7.4 version software.

SUMMARY

Any Tall building can vibrate in both the directions of along wind and across wind caused by the flow of wind. Modern Tall buildings designed to satisfy lateral drift requirements, still may oscillate excessively during wind storm. These oscillations can cause some threats to the Tall building as buildings with more and more height becomes more vulnerable to oscillate at high speed winds. Sometimes these oscillations may even cause discomfort to the occupants even if it is not in a threatening position for the structural damage. So an accurate assessment of building motion is an essential prerequisite for serviceability.

II.LITERATURE REVIEW

Ning Lina, Chris Letchforda, Yukio Tamurab, Bo Liang and Osamu Nakamurad

These papers studied nine models with different rectangular cross -sections and were tested in a wind tunnel to study the characteristics of wind forces on tall buildings. The data was briefly reported (Local wind forces acting on rectangular prisms. Proceedings of 14th National Symposium on Wind Engineering, 4–6 December 1996, Japan Association for Wind Engineering, Tokyo, pp. 263–268.). In the present paper, local wind forces on tall buildings are investigated in terms of mean and RMS force coefficients, power spectral density, and span wise correlation and coherence. The effects of three parameters, elevation, aspect ratio, and side ratio, on bluff -body flow and thereby on the local wind forces are discussed. The overall loads and base moments are obtained by integration of local wind forces. Comparisons are made with results obtained from high –frequency force balances in two wind tunnels.

Holmes and Lewis (1986, 1987 and 1989)

They performed extensive experimental work on the fluctuating pressure measurements using a small diameter connecting tube to transmit the pressure from the connecting point, or tap, to the pressure transducer. Their authentic work has provided sufficient guidelines to develop a range near optimum systems for the measurement of fluctuating pressure on models of the buildings in wind tunnels. In the present study the choice of tubing system for pressure measurements is largely based on the work of Holmes and Lewis (1987).

III.EFFECT OF WIND LOAD ON BUILDINGS AND STRUCTURES**BASIC WIND SPEED:**

Figure 1 gives basic wind speed map of India, as applicable at 10 m height above mean ground level for different zones of the country. Basic wind speed is based on peak gust speed averaged over a short time interval of about 3 seconds and corresponds to 10m height above the mean ground level in an open terrain (Category 2). Basic wind speeds presented in Fig. 1 have been worked out for a 50-year return period. The basic wind speed for some important cities/towns is also given in Appendix A.

Wind load calculations as per the code is 875 : 1987**Design Wind Speed (V_z)**

The design wind speed, V_z at any height, Z for the chosen structure: (a) Risk level, (b) Terrain roughness and height of structure, (c) Local topography, and (d) Importance factor for the cyclonic region. It can be mathematically expressed as follows:

$$V_z = V_b K_1 K_2 K_3 K_4$$

Where V_z = design wind speed at any height z in m/s,

k_1 = probability factor (risk coefficient)

k_2 = terrain roughness and height factor

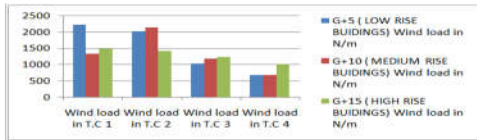
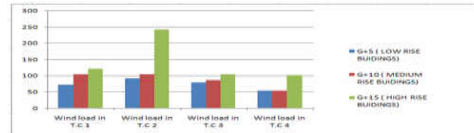
k_3 = topography factor

k_4 = importance factor for the cyclonic region

Design wind pressure ; $P_z = 0.6 V_z^2$

COMPARASION OF WIND LOADS AS PER THE IS 875:2015

S.NO	Terrain category	G+5 (LOW RISE BUIDINGS) Wind load in N/m	G+10 (MEDIUM RISE BUIDINGS) Wind load in N/m	G+15 (HIGH RISE BUIDINGS) Wind load in N/m
1	Wind load in T.C 1	2216.525	1329.92	1483.248
2	Wind load in T.C 2	2014.214	2134.44	1405.536
3	Wind load in T.C 3	1026.389	1180.685	1232.342
4	Wind load in T.C 4	670.94	670.94	1004.667



WIND LOAD CALCUCATIONS AS PER THE CODE IS 875:2015

Design of wind speed as per the 2015 code

$$V_z = V_b k_1 k_2 k_3 k_4 \text{ (AS PER 875 (PART-3) :2015)}$$

V_z = design wind speed at height z , in m/s;

k_1 = probability factor (risk coefficient)

k_2 = terrain roughness and height factor

k_3 = topography factor ; and

k_4 = importance factor for the cyclonic region .

$$P_z = 0.6 V_z^2$$

p_z = wind pressure at height z , in Is N/m², and V_z = design wind speed at height z , in m/s. based on the modified code the wind pressure can be also obtained

The design wind pressure p_d can be obtained as,

$$P_d = K_d K_a K_c P_z$$

K_d = wind directionality factor,

K_a = area averaging factor, and

K_c = combination factor.

COMPARASION OF WIND LOADS AS PER THE IS 875:2015

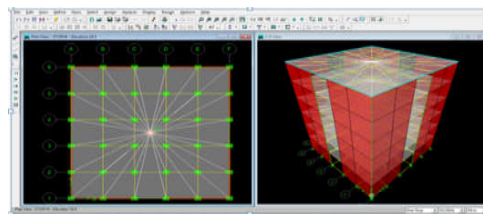
S.NO	Terrain category	G+5 (LOW RISE BUIDINGS)	G+10 (MEDIUM RISE BUIDINGS)	G+15 (HIGH RISE BUIDINGS)
1	Wind load in T.C 1	72.334	104.92	120.44
2	Wind load in T.C 2	92.204	104.92	241.704
3	Wind load in T.C 3	78.6924	85.3134	104.214
4	Wind load in T.C 4	53.54	53.53	101.199

IV.DESIGN CONSIDERATIONS AND MODELING OF BUILDING IN ETABS

LOW RISE BUILDING

G+5 Design details	
Type of structure	RCC frame structure
Number of stories (G+5)	6 Stories
Story to story height	3m
Ground story height	3.5m
Grade of concrete	M30 for columns and slab M25 for Beams
Thickness of slab	0.12m
Thickness of wall	0.23m
Beam size	0.3mX0.4m
Column size	0.4mX0.6m
Density	For concrete 24kN/m ³ For brick wall 19kN/m ³

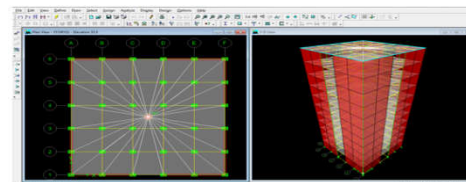
Low Rise Building ((G+5) 4 models



MEDIUM RISE BUILDINGS

G+10 Design details	
Type of structure	RCC frame structure
Number of stories (G+5)	11Stories
Story to story height	3m
Ground story height	3.5m
Grade of concrete	M30 for columns and slab M25 for Beams
Thickness of slab	0.12m
Thickness of wall	0.23m
Beam size	0.3mX0.4m
Column size	0.4mX0.6m
Density	For concrete 24kN/m ³ For brick wall 19kN/m ³

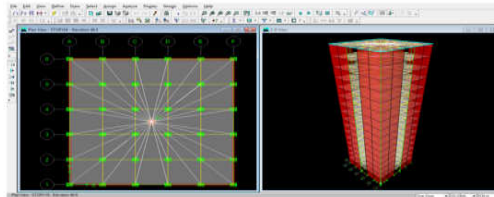
Medium Rise Building ((G+10) 4 models



HIGH RISE BUILDING

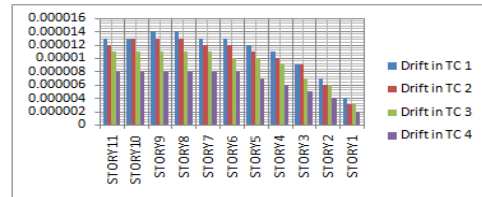
G+15 Design details	
Type of structure	RCC frame structure
Number of stories (G+5)	16Stories
Story to story height	3m
Ground story height	3.5m
Grade of concrete	M30 for columns and slab M25 for Beams
Thickness of slab	0.12m
Thickness of wall	0.23m
Beam size	0.3mX0.4m
Column size	0.4mX0.6m
Density	For concrete 24kN/m ³ For brick wall 19kN/m ³

High Rise Building ((G+15) 4 models)



G+10

Story	Item	Load	Drift in TC 1	Drift in TC 2	Drift in TC 3	Drift in TC 4
STORY11	Max Drift X	WIND	0.000013	0.000012	0.000011	0.000008
STORY10	Max Drift X	WIND	0.000013	0.000013	0.000011	0.000008
STORY9	Max Drift X	WIND	0.000014	0.000013	0.000011	0.000008
STORY8	Max Drift X	WIND	0.000014	0.000013	0.000011	0.000008
STORY7	Max Drift X	WIND	0.000013	0.000012	0.000011	0.000008
STORY6	Max Drift X	WIND	0.000013	0.000012	0.00001	0.000008
STORY5	Max Drift X	WIND	0.000012	0.000011	0.00001	0.000007
STORY4	Max Drift X	WIND	0.000011	0.00001	0.000009	0.000006
STORY3	Max Drift X	WIND	0.000009	0.000009	0.000007	0.000005
STORY2	Max Drift X	WIND	0.000007	0.000006	0.000006	0.000004
STORY1	Max Drift X	WIND	0.000004	0.000003	0.000003	0.000002



V.RESULTS AND ANALYSIS

STORY DRIFT

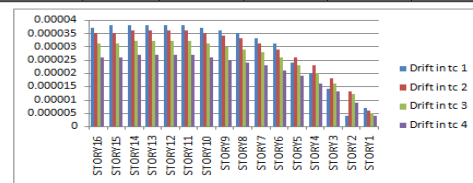
G+5

Story	Drift in TC 1	Drift in TC 2	Drift in TC 3	Drift in TC 4
STORY6	0.000002	0.000002	0.000002	0.000001
STORY5	0.000002	0.000002	0.000002	0.000001
STORY4	0.000002	0.000002	0.000002	0.000001
STORY3	0.000002	0.000002	0.000002	0.000001
STORY2	0.000002	0.000002	0.000002	0.000001
STORY1	0.000001	0.000001	0.000001	0.000001



G+15

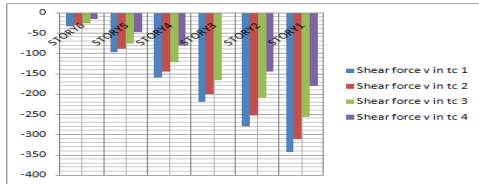
Story	Load	Drift in tc 1	Drift in tc 2	Drift in tc 3	Drift in tc 4
STORY16	WINDX	0.000037	0.000035	0.000031	0.000026
STORY15	WINDX	0.000038	0.000035	0.000031	0.000026
STORY14	WINDX	0.000038	0.000036	0.000032	0.000027
STORY13	WINDX	0.000038	0.000036	0.000032	0.000027
STORY12	WINDX	0.000038	0.000036	0.000032	0.000027
STORY11	WINDX	0.000038	0.000036	0.000032	0.000027
STORY10	WINDX	0.000037	0.000035	0.000031	0.000026
STORY9	WINDX	0.000036	0.000034	0.00003	0.000025
STORY8	WINDX	0.000035	0.000033	0.000029	0.000024
STORY7	WINDX	0.000033	0.000031	0.000028	0.000023
STORY6	WINDX	0.000031	0.000029	0.000026	0.000021
STORY5	WINDX	0.000024	0.000026	0.000023	0.000019
STORY4	WINDX	0.00002	0.000023	0.00002	0.000016
STORY3	WINDX	0.000014	0.000018	0.000016	0.000013
STORY2	WINDX	0.000004	0.000013	0.000012	0.000009
STORY1	WINDX	0.000007	0.000006	0.000005	0.000004



SHEAR FORCE (V)

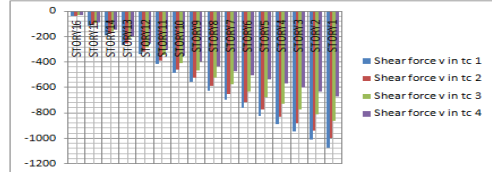
G+5

Story	Shear force v in tc 1	Shear force v in tc 2	Shear force v in tc 3	Shear force v in tc 4
STORY6	-33.06	-30.08	-25.88	-16.22
STORY5	-97.65	-88.79	-75.81	-48.66
STORY4	-159.57	-144.96	-122.33	-81.1
STORY3	-219.42	-199.15	-166.17	0
STORY2	-279	-253.09	-209.66	-145.97
STORY1	-343.55	-311.52	-256.77	-181.11



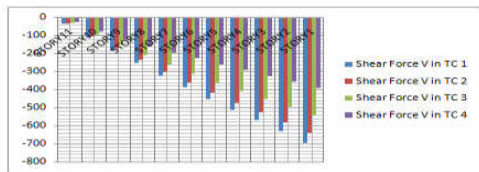
G+15

Story	Load	Shear force v in tc 1	Shear force v in tc 2	Shear force v in tc 3	Shear force v in tc 4
STORY16	WINDX	-38.69	-36.73	-32.9	-30.08
STORY15	WINDX	-115.4	-109.56	-97.99	-88.86
STORY14	WINDX	-191.13	-181.43	-161.98	-145.58
STORY13	WINDX	-265.89	-252.35	-224.91	-200.29
STORY12	WINDX	-339.68	-322.33	-286.77	-253.03
STORY11	WINDX	-412.5	-391.37	-347.57	-303.82
STORY10	WINDX	-484.36	-459.39	-407.26	-352.39
STORY9	WINDX	-555.12	-525.8	-465.4	-396.64
STORY8	WINDX	-624.75	-590.41	-521.84	-435.96
STORY7	WINDX	-693.24	-653.2	-576.59	-470.73
STORY6	WINDX	-760.06	-714.06	-629.2	-503.26
STORY5	WINDX	-824.66	-772.77	-679.13	-535.7
STORY4	WINDX	-886.58	-828.94	-725.65	-568.14
STORY3	WINDX	-946.43	-883.13	-769.49	-600.57
STORY2	WINDX	-1006.01	-937.06	-812.98	-633.01
STORY1	WINDX	-1070.56	-995.5	-860.09	-668.15



G+10

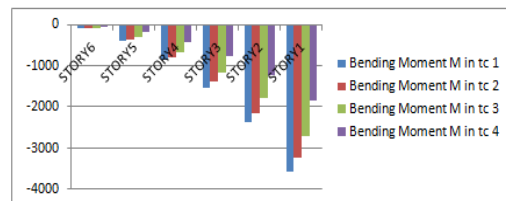
Story	Load	Shear Force V in TC 1	Shear Force V in TC 2	Shear Force V in TC 3	Shear Force V in TC 4
STORY11	WIND	-36.25	-34.36	-30.23	-25.08
STORY10	WIND	-108.1	-102.38	-89.91	-73.64
STORY9	WIND	-178.87	-168.8	-148.06	-117.89
STORY8	WIND	-248.5	-233.4	-204.5	-157.22
STORY7	WIND	-316.99	-296.2	-259.25	-191.98
STORY6	WIND	-383.81	-357.06	-311.85	-224.51
STORY5	WIND	-448.41	-415.77	-361.79	-256.95
STORY4	WIND	-510.33	-471.93	-408.31	-289.39
STORY3	WIND	-570.18	-526.13	-452.14	-321.83
STORY2	WIND	-629.76	-580.06	-495.63	-354.26
STORY1	WIND	-694.3	-638.49	-542.75	-389.41



BENDING MOMENT (M)

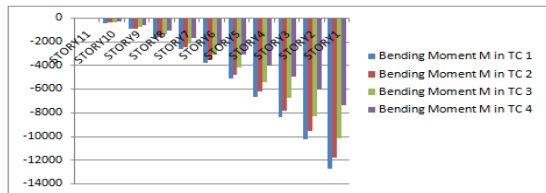
G+5

story	Bending Moment M in tc 1	Bending Moment M in tc 2	Bending Moment M in tc 3	Bending Moment M in tc 4
STORY6	-99.17	-90.24	-77.637	-48.657
STORY5	-392.117	-356.611	-305.066	-194.628
STORY4	-870.835	-791.483	-672.056	-437.913
STORY3	-1529.1	-1388.93	-1170.56	-778.512
STORY2	-2366.11	-2148.19	-1799.53	-1216.43
STORY1	-3568.52	-3238.5	-2698.23	-1850.32



G+10

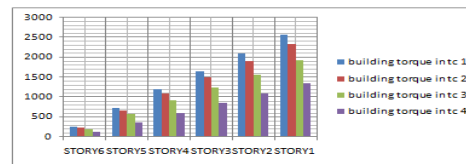
Story	Load	Bending Moment M in TC 1	Bending Moment M in TC 2	Bending Moment M in TC 3	Bending Moment M in TC 4
STORY11	WIND	-108.759	-103.092	-90.676	-75.229
STORY10	WIND	-433.073	-410.242	-360.416	-296.144
STORY9	WIND	-969.674	-916.651	-804.589	-649.829
STORY8	WIND	-1715.17	-1616.86	-1418.09	-1121.48
STORY7	WIND	-2666.14	-2505.47	-2195.83	-1697.42
STORY6	WIND	-3817.58	-3576.64	-3131.4	-2370.96
STORY5	WIND	-5162.8	-4823.94	-4216.76	-3141.81
STORY4	WIND	-6693.79	-6239.75	-5441.67	-4009.98
STORY3	WIND	-8404.33	-7818.13	-6798.1	-4975.46
STORY2	WIND	-10293.6	-9558.32	-8285	-6038.25
STORY1	WIND	-12723.7	-11793	-10184.6	-7401.17



BUILDIND TORQUE (T):

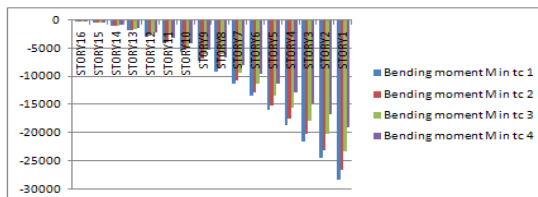
G+5

Story	building torque in tc 1	building torque in tc 2	building torque in tc 3	building torque in tc 4
STORY6	247.924	225.601	194.092	121.643
STORY5	732.369	665.928	568.574	364.928
STORY4	1196.794	1087.18	917.474	608.213
STORY3	1645.665	1493.622	1246.249	851.498
STORY2	2092.516	1898.143	1572.426	1094.783
STORY1	2576.605	2336.373	1925.785	1358.342



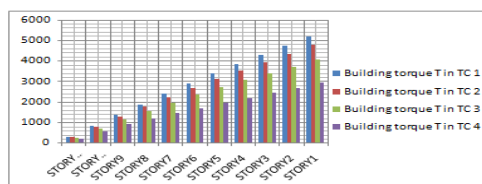
G+15

Story	Load	Bending moment M in tc 1	Bending moment M in tc 2	Bending moment M in tc 3	Bending moment M in tc 4
STORY16	WINDX	-116.056	-110.199	-98.713	-90.24
STORY15	WINDX	-462.254	-438.877	-392.671	-356.809
STORY14	WINDX	-1035.65	-983.165	-878.624	-793.555
STORY13	WINDX	-1833.32	-1740.21	-1553.35	-1394.44
STORY12	WINDX	-2852.35	-2707.19	-2413.65	-2153.53
STORY11	WINDX	-4089.86	-3881.29	-3413.65	-3065
STORY10	WINDX	-5542.92	-5259.44	-4678.12	-4122.16
STORY9	WINDX	-7208.28	-6836.86	-6074.32	-5312.09
STORY8	WINDX	-9082.53	-8608.07	-8074.32	-6619.98
STORY7	WINDX	-11162.2	-10567.7	-9369.62	-8032.17
STORY6	WINDX	-13442.4	-12709.9	-11257.2	-9541.95
STORY5	WINDX	-15916.4	-15028.2	-13294.6	-11149
STORY4	WINDX	-18576.1	-17515	-15471.5	-12853.4
STORY3	WINDX	-21415.4	-20164.4	-17780	-14655.2
STORY2	WINDX	-24433.5	-22975.6	-20218.9	-16554.2
STORY1	WINDX	-28180.4	-26459.8	-23229.2	-18892.7



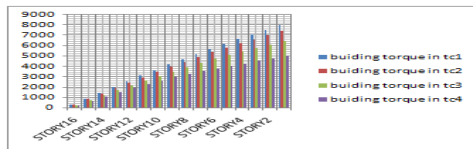
a. G+10

Story	Load	Building torque T in TC 1	Building torque T in TC 2	Building torque T in TC 3	Building torque T in TC 4
STORY11	WIND	271.897	257.729	226.69	188.072
STORY10	WIND	810.785	767.876	674.35	552.288
STORY9	WIND	1341.504	1266.021	1110.432	884.211
STORY8	WIND	1863.748	1750.525	1533.757	1179.122
STORY7	WIND	2377.41	2221.516	1944.354	1439.867
STORY6	WIND	2878.604	2677.93	2338.912	1683.841
STORY5	WIND	3363.049	3118.257	2713.394	1927.127
STORY4	WIND	3827.474	3539.509	3062.295	2170.412
STORY3	WIND	4276.345	3945.952	3391.07	2413.697
STORY2	WIND	4723.196	4350.472	3717.247	2656.982
STORY1	WIND	5207.285	4788.703	4070.606	2920.541



G+15

Story	Load	Building torque in tc1	Building torque in tc2	Building torque in tc3	building torque in tc4
STORY16	WINDX	290.14	275.499	246.782	225.601
STORY15	WINDX	865.494	821.694	734.896	666.421
STORY14	WINDX	1433.487	1360.719	1214.883	1091.867
STORY13	WINDX	1994.166	1892.62	1686.81	1502.209
STORY12	WINDX	2547.58	2417.445	2150.747	1897.722
STORY11	WINDX	3093.776	2935.242	2606.761	2278.679
STORY10	WINDX	3632.663	3445.389	3054.42	2642.895
STORY9	WINDX	4163.383	3943.534	3490.503	2974.819
STORY8	WINDX	4685.627	4428.038	3913.827	3269.729
STORY7	WINDX	5199.289	4899.029	4324.424	3530.474
STORY6	WINDX	5700.483	5355.442	4718.982	3774.449
STORY5	WINDX	6184.928	5795.77	5093.465	4017.734
STORY4	WINDX	6649.353	6217.022	5442.365	4261.019
STORY3	WINDX	7098.224	6623.464	5771.14	4504.304
STORY2	WINDX	7545.075	7027.984	6097.317	4747.589
STORY1	WINDX	8029.163	7466.215	6450.676	5011.148



V.CONCLUSIONS

From the above study the following conclusions were made

1. The values of storey drifts are constant in G+5 building design in all terrain categories up to 2nd storey and is decreases to 1st storey this indicates there is less effect of wind in the low rise buildings
2. In case of medium rise and high rise buildings value of story drift is decreases from top story to bottom story (11th to 1st in medium rise buildings and 16th to 1st in High rise buildings). And the Higher drift values are obtained in terrain category 1 and lower drift values are obtained at terrain category 4.
3. The maximum values of building torque (T) was obtained terrain category 1 than remaining terrains. The value of building twist decreases from 6th storey to 1st story.
4. The maximum values of Shear forces and Bending moments are obtained at terrain category 1. The forces and moments are decreases from top story to bottom storey (6th to 1st in case of low rise buildings, 11th

to 1st in medium rise buildings and 16th to 1st in High rise buildings)

5. For the above conclusions the maximum values are obtained at terrain category 1 in all cases and minimum values are obtained in terrain category 4 from this it was concluded that there is no wind effect on buildings which are in terrain category 4 than other terrain categories.

REFERENCES:

- [1] A. Tallin, B. Ellingwood.” Wind-induced motion of tall buildings ”.Engineering Structures, Volume 7, Issue 4, October 1985,
- [2] Blackmore,P.A.(1985). "A Comparison of Experimental Methods for Estimating Dynamic Response of Buildings." J. W.E. & I.A. , 18, 197-212.
- [3] BIS (1987). Indian Standards Code of Practice for Design Loads (Other than Earthquake) for Buildings and Structures pt.3 - Wind Loads. Bureau of Indian Standards, India.
- [4] Balendra,T., Nathan,G.K., and Kang,K.H.(1989). "A Deterministic Model for Alongwind Motion of Buildings." J. Engrg. Structures, 11, 16 -22.
- [5] Balendra,T., Tan,C L., and Ma, Z. (2003). “Design of Tall Residential Building In Singapore For Wind Effects.” *Wind and Structures Vol.6*,.221-248.
- [6] Cermak,J.E.(1977). "Wind Tunnel Testing of Structures." J. Engrg. Mech., ASCE, 103(EM6), 1125-1140.
- [7] Cermak,J.E.(1979). "Applications of Wind Tunnels to Investigation of Wind Engineering Problems." J. AIAA, 17(7), 679 -690.
- [8] Cermak,J.E.(1981). "Wind Tunnel Design for Physical Modelling of Atmospheric Boundary Layer." J. Engrg. Mech., ASCE, 107(EM3), 623 -640.
- [9] Cermak,J.E.(1982). "Physical Modelling of the Atmospheric Boundary Layer in Long Boundary Layer Wind Tunnels." Proc. Int. W/S on Wind Tunnel Modelling, USA, 97 - 125.